RECEIVED

APR 0 7 2021

INLAND WETLANDS & WATERCOURSES COMMISSION TOWN OF BROOKLYN, CONECTICUT

Date	04 13 215
Date	Application #

APPLICATION -- INLAND WETLANDS & WATERCOURSES

APPLICANT A. VAISCH & SONS MAILING ADDRESS 15 BEACH VIEW PO EXT, VOICITIONS APPLICANT'S INTEREST IN PROPERTY OPEN PHONE EMAIL
PROPERTY OWNER IF DIFFERENT PHONE MAILING ADDRESS EMAIL
ENGINEER/SURVEYOR (IF ANY) - ARCHAR SURVEYORS LLC. CLA BUGNESONS
PROPERTY LOCATION/ADDRESS PONTIES (MID JG PD) (HINCH STREET) MAP # 30 LOT # 20/21 ZONE PA TOTAL ACRES 29 ACRES OF WETLANDS ON PROPERTY 6 1 PURPOSE AND DESCRIPTION OF THE ACTIVITY WETLANDS (NUSSIN/G FIN DRIVEWAY) 2 RESIDENT HOMES SEPTIC SYSTEM WELL - Migh Employed
WETLANDS EXCAVATION AND FILL: FILL PROPOSED CUBIC YDS SQ FT 3, 650 SQFT SQ FT
MITIGATION MEASURES (IF REQUIRED): WETLANDS/WATERCOURSES CREATED: CY SQFT ACRES
IS PARCEL LOCATED WITHIN 500FT OF AN ADJOINING TOWN? IF YES, WHICH TOWN(S) IS THE ACTIVITY LOCATED WITHIN THE WATERSHED OF A WATER COMPANY AS DEFINED IN CT GENERAL STATUTES 25-32A?
THE OWNER AND APPLICANT HEREBY GRANT THE BROOKLYN IWWC, THE BOARD OF SELECTMAN AND THEIR AUTHORIZED AGENTS PERMISSION TO ENTER THE SUBJECT PROPERTY FOR THE PURPOSE OF INSPECTION AND ENFORCEMENT OF THE IWWC REGULATIONS OF THE TOWN OF BROOKLYN. IF THE COMMISSION DETERMINES THAT OUTSIDE REVIEW IS REQUIRED, APPLICANT WILL PAY CONSULTING FEE.
NOTE: DETERMINATION THAT THE INFORMATION PROVIDED IS INACCURATE MAY INVALIDATE THE IWWC DECISION AND RESULT IN ENFORCEMENT ACTION. APPLICANT: DATE 4(5/2) DATE 4(5/2)
OWNER: DATE 4/5/2



GIS CODE #:					_	_		
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79 Elm Street • Hartford, CT 06106-5127

www.ct.gov/deep

Affirmative Action/Equal Opportunity Employer

Statewide Inland Wetlands & Watercourses Activity Reporting Form

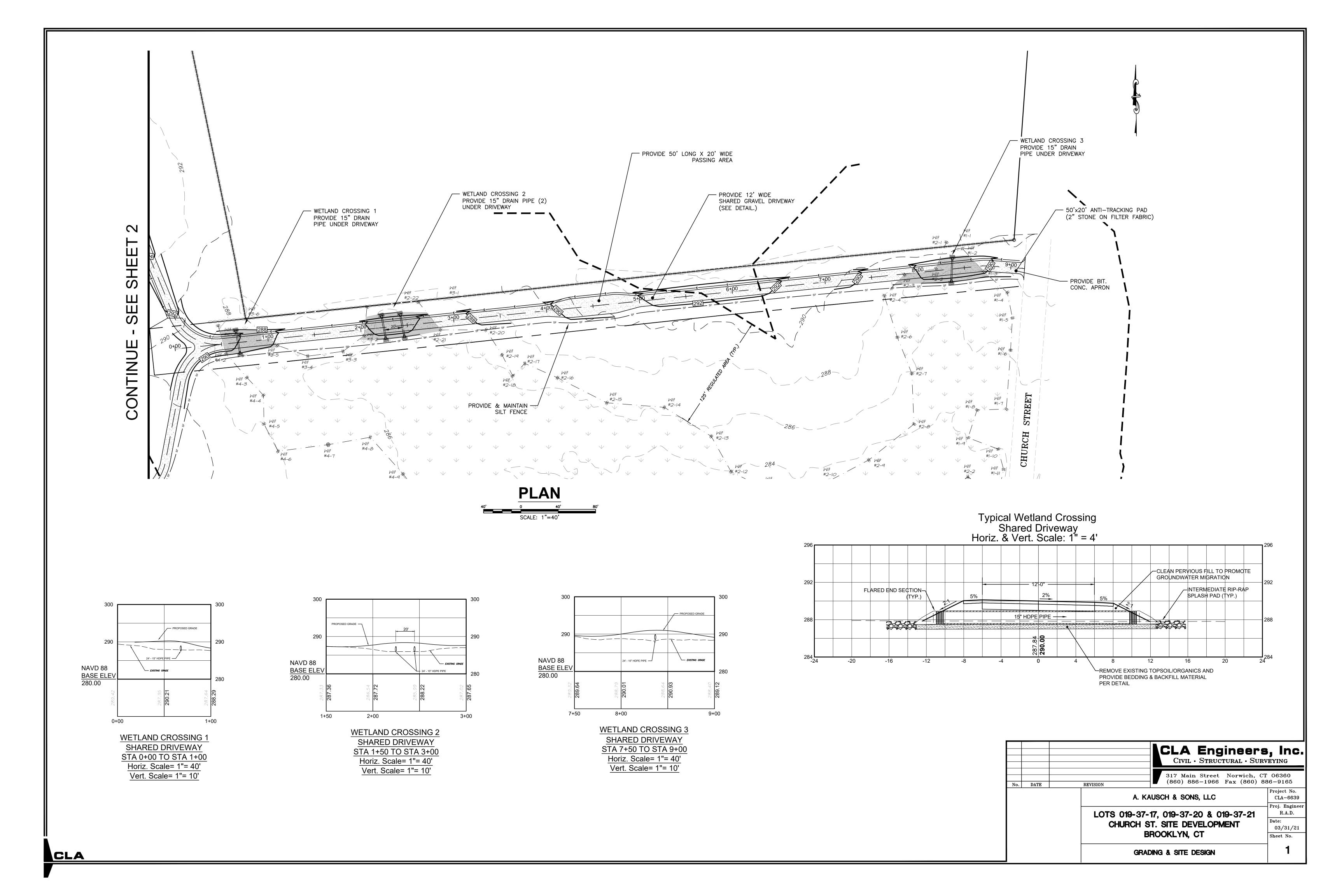
Please complete and mail this form in accordance with the instructions on pages 2 and 3 to:

DEEP Land & Water Resources Division, Inland Wetlands Management Program, 79 Elm Street, 3rd Floor, Hartford, CT 06106

Incomplete or incomprehensible forms will be mailed back to the inland wetlands agency.

	PART I: Must Be Completed By The Inland Wetlands Agency
1.	DATE ACTION WAS TAKEN: year: month:
2,	ACTION TAKEN (see instructions, only use one code):
3.	WAS A PUBLIC HEARING HELD (check one)? yes no
4	NAME OF AGENCY OFFICIAL VERIFYING AND COMPLETING THIS FORM:
	(print name) (signature)
	(Signature)
	PART II: To Be Completed By The Inland Wetlands Agency Or The Applicant
5.	TOWN IN WHICH THE ACTION IS OCCURRING (print name):
Thromballabara (Mariana)	does this project cross municipal boundaries (check one)? yes no
6.	if yes, list the other town(s) in which the action is occurring (print name(s)): LOCATION (see instructions for information): USGS quad name:
0.	subregional drainage basin number:
7	NAME OF APPLICANT, VIOLATOR OR PETITIONER (print name): A. KASCH & Sons
8.	NAME & ADDRESS / LOCATION OF PROJECT SITE (print information): Cthort ST / Banky A'D Pontage (page)
	briefly describe the action/project/activity (check and print information): temporary permanent description:
	Dasserry, Dosiderithe things
9.	ACTIVITY PURPOSE CODE (see instructions, only use one code):
10.	. ACTIVITY TYPE CODE(S) (see instructions for codes):,
11.	. WETLAND / WATERCOURSE AREA ALTERED (must provide acres or linear feet):
	wetlands: _ 4 © B acres open water body:acres stream:linear feet
12.	. UPLAND AREA ALTERED (must provide acres): acres
13	. AREA OF WETLANDS / WATERCOURSES RESTORED, ENHANCED OR CREATED (must provide acres):acres
D	ATE RECEIVED: PART III: To Be Completed By The DEEP DATE RETURNED TO DEEP:
F	ORM COMPLETED: YES NO FORM CORRECTED / COMPLETED: YES NO

REQUIREMENT	<u>S</u>			+	
Аррі	LICATION FEE \$ 150	STATE FEE (\$60.00)	60	61C	
Cov	IPLETION OF CT DEEP REPORTI	ING FORM			
ORIG	GINAL PLUS COPIES OF ALL MATI	ERIALS REQUIRED - NUMBE	ER TO BE DETERMINED	BY STAFF	
PRE	-APPLICATION MEETING WITH T	HE WETLANDS AGENT IS RI	ECOMMENDED TO EXA	MINE THE SCOPE OF THE A	CTIVITY
	PLAN SHOWING LOCATION OF IED SOIL SCIENTIST IDENTIFY TH		ING AND PROPOSED C	ONDITIONS. APPLICANT M	AY BE REQUIRED
Con	APLIANCE WITH THE CONNECTION	CUT EROSION & SEDIMENT	ATION CONTROL MAN	NUAL	
FOLLOWING INFO	HE PROPOSED ACTIVITY IS DEEM	ED TO BE A "SIGNIFICANT II	MPACT ACTIVITY" A PI	UBLIC HEARING IS REQUIRE	D ALONG WITH THE
-	NAMES AND ADDRESSES OF A ADDITIONAL INFORMATION A			7.6	
ADDITIONALIN	IFORMATION/ACTION NEE	DED:			
		-			
DEPARTME	INLAND WATER RESOURCES DIVISION 79 ELM ST. HARTFORD, CT. 06106 1-860-424-3019 ENT OF THE ARMY CORPS OF ENGINEERS 696 VIRGINIA ROAD CONCORD, MA. 01742 1-860-343-4789				
STAFF USE ONLY:					
DECLA	RATORY RULING: AS OF RIGHT	& Non-Regulated Uses	(SEE IWWC REGULA	TIONS SECTION 4)	
PERMI	IT REQUIRED: _AUTHORIZED BY STAFF/CHAII	R (NO ACTIVITY IN WETLAN	DS/WATERCOURSE AN	ID MINIMAL IMPACT)	
	CHAIR, BROOKLYN IWWC _AUTHORIZED BY IWWC _SIGNIFICANT ACTI	IVITY/PUBLIC HEARING	WETLANDS OFFICER		
No pe	RMIT REQUIRED	·			
	_ outside of upland review _ no impact	/ AREA			
	CHAIR, BROOKLYN IWWC		WETLANDS OFFICER		
Тімве	er Harvest				



SELECT FILL SPECIFICATION

SELECT FILL PLACED WITHIN AND ADJACENT TO LEACHING SYSTEM AREAS SHALL BE COMPRISED OF CLEAN SAND, OR SAND AND GRAVEL, FREE FROM ORGANIC MATTER AND FOREIGN SUBSTANCES. THE SELECT FILL SHALL MEET THE FOLLOWING REQUIREMENTS PER THE CONNECTICUT PUBLIC HEALTH

1. THE SELECT FILL SHALL NOT CONTAIN ANY MATERIAL LARGER THAN THE THREE (3) INCH SLEEVE.

2. UP TO 45% OF THE DRY WEIGHT OF THE REPRESENTATIVE SAMPLE MAY BE RETAINED ON THE #4 SLEEVE (THIS IS THE GRAVEL PORTION OF THE

- 3. THE MATERIAL THAT PASSES THE #4 SIEVE IS THEN REWEIGHED AND THE SIEVE ANALYSIS STARTED,
 4. THE REMAINING SAMPLE SHALL MEET THE FOLLOWING CRITERIA:
- PERCENT PASSING WET SIEVE DRY SIEVE

70-100 70-100 10-50* 10-75 0-20 0 - 2.5

* PERCENT PASSING THE #40 SIEVE CAN BE INCREASED TO NO GREATER THAN 75 IF THE PERCENT PASSING THE #100 SIEVE DOES NOT EXCEED 10 AND THE #200 SIEVE DOES NOT EXCEED 5.

SEPTIC NOTES

- PROPOSED SEPTIC SYSTEM TO BE STAKED IN THE FIELD BY A LAND SURVEYOR LICENSED IN THE STATE OF CONNECTICUT.
- 2. A BENCHMARK SHALL BE SET WITHIN 10'-15' OF THE PROPOSED SEPTIC SYSTEM PRIOR TO CONSTRUCTION. ALL WORK AND MATERIAL (SEPTIC TANK, DISTRIBUTION BOX, PIPE) SHALL CONFORM TO THE CONNECTICUT PUBLIC HEALTH CODE REGULATIONS AND
- STANDARDS FOR SUBSURFACE SEWAGE DISPOSAL SYSTEM. SEWER LINE FROM FOUNDATION WALL TO SEPTIC TANK SHALL BE 4" SCHEDULE 40 PVC - ASTM D 1785 AND JOINTS PER HEALTH DEPT. CODE.
- PIPE FROM SEPTIC TANK TO DISTRIBUTION LINES SHALL BE 4" SOLID PVC CONFORMING TO STMD-3034 AND SDR-35. SYSTEMS SHALL BE SET LEVEL FOR ENTIRE LENGTH AND HAVE A CENTER TO CENTER SPACING AS CALLED FOR IN THE CONNECTICUT PUBLIC
- HEALTH CODE. THERE ARE PRESENTLY NO KNOWN WATER WELLS WITHIN 75' OF THE PROPOSED SEPTIC SYSTEMS.

 5. CLEAR AND GRUB THE AREA WHERE THE SEPTIC SYSTEMS AND HOUSES ARE TO BE CONSTRUCTED. ALL TOPSOIL IS TO BE STRIPPED AND STOCKPILED FOR FUTURE USE.

 7. ALL FILL MATERIAL SHALL BE CLEAN EARTH FREE OF STUMPS, ORGANICS, CONSTRUCTION DEBRIS AND TOPSOIL.

 8. TOPSOIL SHALL BE RE—APPLIED OVER ALL FILL AREAS AND ALL DISTURBED AREAS TO PROVIDE A MINIMUM DEPTH OF FOUR INCHES IN
- ACCORDANCE WITH THE SLOPE STABILIZATION DETAILS..

WITNESSED BY:NORTHEAST DISTRICT DEPARTMENT OF HEALTH DATE: 03/30/2021					
TEST PIT: 1	TEST PIT: 2				
0" - 10" Topsoil / Organics 10" - 16" Orange Brown Fine Sand Loam 16" - 34" Tan Compact Sand Loam 34" - 78" Grey Sand & Gravel	0" - 10" Topsoil / Organics 10" - 27" Dark Brown Fine Sand 27" - 48" Grey Compact Sand 48" - 70" Sand & Gravel				
MOTTLES: 36"	MOTTLES: 32"				
GROUNDWATER: 66"	GROUNDWATER: 62"				
LEDGE: NO	LEDGE: NO				

TIME	(INCHES)			
11:10	6.0			
11:12	7.0			
11:14	8.0			
11:16	8.5			
11:18	9.25			
11:20	9.5			
11:22	10.0			
11:25	11.0			
11:28	11.5			
11:31	12.0			
PERCOLATION RATE > 6.0 MIN./IN.				
NOTES: PERCOLATION TEST PERFORMED ON 3/30/2021 PERFORMED BY Donovan Moe				

PERCOLATION DATA

PERC 1 - DEPTH 23"

	ROOTS: 46"	ROOTS: 32"
	RESTRICTIVE: NO	RESTRICTIVE: NO
,		
	TEST PIT: 3	TEST PIT: 4
	0" - 12" Topsoil / Organics 12" - 30" Tan Orange Fine Sand Loam 30" - 48" Grey Sand Layer 48" - 72" Sand & Gravel	0" - 10" Topsoil / Organics 10" - 24" Tan Orange Fine Sand Loam 24" - 80" Saturated Grey Sand & Gravel
	MOTTLES: 30"	MOTTLES: 28"
	GROUNDWATER: 65"	GROUNDWATER: 74"
	LEDGE: NO	LEDGE: NO
	ROOTS: NO	ROOTS: 30"
	RESTRICTIVE: NO	RESTRICTIVE: NO

PERCOLATION DATA				
PERC 2	PERC 2 - DEPTH 17"			
TIME	DROP (INCHES)			
11:04	6.25			
11:10	8.25			
11:16	9.75			
11:22	10.625			
11:34	12.125			
11:46	13.125			
11:58	14.625			
PERCOLATION RATE > 8.0 MIN./IN.				

CONCEPT SEPTIC SYSTEM DESIGN

PROPOSED LOT 1
PRIMARY LEACHING AREA
4 BEDROOM RESIDENCE

PERCOLATION RATE: 6.0 MIN./INCH (NDDH FILE #21000307)
LEACHING AREA REQUIRED: 557.5 SF

USE TRADITIONAL TRENCH

EFFECTIVE LEACHING AREA OF LEACHING TRENCH 3.0 SF/LF REQUIRED LENGTH = 557.5 SF / 3 SF/LF = 186 LF

DEPTH TO RESTRICTIVE LAYER = 32" SLOPE = 5.0%

HYDRAULIC FACTOR (HF) = 32FLOW FACTOR (FF) = 1.0

PERCOLATION FACTOR (PF) = 1.75 (LESS THAN 10.0 MIN./INCH) MLSS REQUIRED: $32 \times 1.0 \times 1.75 = \underline{52.5 \text{ LF}}$

PROPOSED SYSTEM

USE 3 ROWS OF 65 LF

LEACHING AREA PROVIDED = 585 SF

RESERVE LEACHING AREA USE SAME AS PRIMARY SYSTEM

PROPOSED LOT 2
PRIMARY LEACHING AREA
4 BEDROOM RESIDENCE

PERCOLATION RATE: 8.0 MIN./INCH (NDDH FILE #21000307)
LEACHING AREA REQUIRED: 557.5 SE

USE TRADITIONAL TRENCH

EFFECTIVE LEACHING AREA OF LEACHING TRENCH 3.0 SF/LF REQUIRED LENGTH = 557.5 SF / 3 SF/LF = 186 LF

MLSS CALCULATION HYDRAULIC FACTORS

DEPTH TO RESTRICTIVE LAYER = 28"

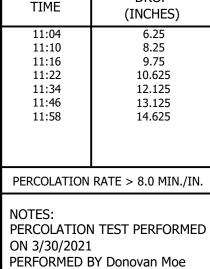
SLOPE = 4.0%HYDRAULIC FACTOR (HF) = 34

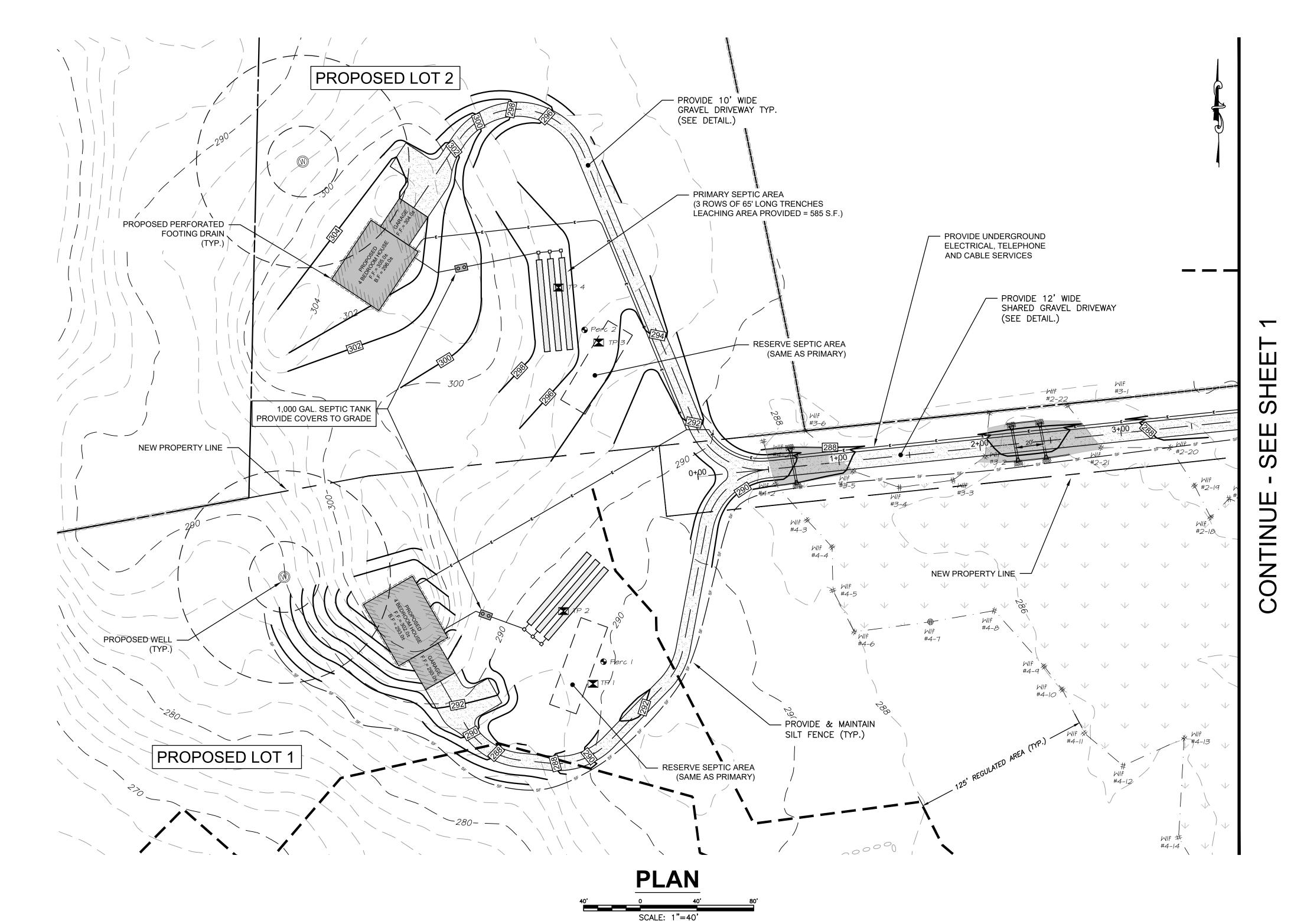
FLOW FACTOR (FF) = 1.0

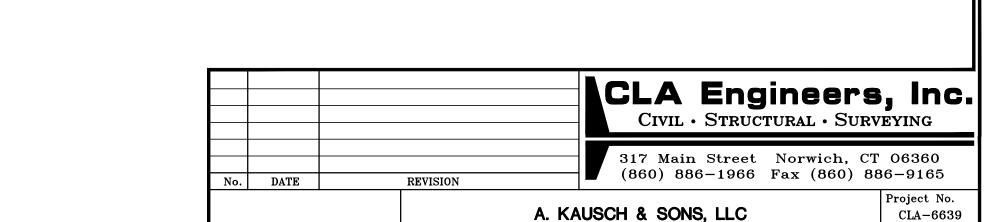
PERCOLATION FACTOR (PF) = 1.75 (LESS THAN 10.0 MIN./INCH) MLSS REQUIRED: $34 \times 1.0 \times 1.75 = \underline{59.5 \text{ LF}}$

PROPOSED SYSTEM
USE 3 ROWS OF 65 LF

LEACHING AREA PROVIDED = 585 SF







LOTS 019-37-17, 019-37-20 & 019-37-21 CHURCH ST. SITE DEVELOPMENT BROOKLYN, CT

Proj. Engineer

R.A.D.

03/31/21

Sheet No.

GRADING & SITE DESIGN

CLA

EROSION & SEDIMENTATION CONTROL NARRATIVE

- 1. THE EROSION & SEDIMENTATION CONTROL PLAN AND DETAILS HAVE BEEN DEVELOPED AS A STRATEGY TO CONTROL SOIL EROSION AND SEDIMENTATION DURING AND AFTER CONSTRUCTION. THIS PLAN IS BASED ON THE "2002 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL" BY THE CONNECTICUT COUNCIL ON SOIL AND WATER CONSERVATION IN COOPERATION WITH THE
- CONNECTICUT DEP. 2. THE PROPOSED LOCATIONS OF SILTATION AND EROSION CONTROL MEASURES ARE SHOWN ON THE PLANS. THE CONTRACTOR SHALL PROVIDED SILT FENCE, STONE CHECK DAMS AND/OR OTHER EROSION CONTROL MEASURES AS NEEDED OR DIRECTED BY THE ENGINEER OR TOWN STAFF TO ADEQUATELY PREVENT SEDIMENT TRANSPORT.
- EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO SITE DISTURBANCE.
- 4. THE CONTRACTOR SHALL INSPECT, REPAIR AND/OR REPLACE EROSION CONTROL MEASURES EVERY 7 DAYS AND IMMEDIATELY FOLLOWING ANY SIGNIFICANT RAINFALL OR SNOW MELT. SEDIMENT DEPOSITS MUST BE REMOVED WHEN WHEN DEPOSITS REACH APPROXIMATELY ONE HALF THE HEIGHT OF THE BARRIER. SEDIMENT CONTROL DEVICES SHALL REMAIN IN PLACE AND BE MAINTAINED BY THE
- CONTRACTOR UNTIL AREAS UPSLOPE ARE PERMANENTLY STABILIZED. 5. STAKED HAY BALE SILT BARRIERS OR SILT FENCE SHALL BE INSTALLED AROUND ANY TEMPORARY
- STOCKPILE AREAS. TEMPORARY VEGETATIVE COVER MAY BE REQUIRED (SEE NOTE). 6. INLET SEDIMENTATION CONTROL DEVICES SHALL BE INSTALLED UNDER THE GRATES OF ALL NEW CATCH BASINS AT THE TIME OF INSTALLATION, AND UNDER THE GRATES OF EXISTING CATCH BASINS
- 7. CONTINUOUS DUST CONTROL USING WATER, CALCIUM CHLORIDE OR APPROVED EQUAL SHALL BE PROVIDED FOR ALL EARTH STOCKPILES, EARTH PILED ALONG EXCAVATIONS, SURFACES OF BACKFILLED TRENCHES AND GRAVELED ROADWAY SURFACES.
- 8. IF DEWATERING IS NECESSARY DURING ANY TIME OF CONSTRUCTION A CLEAR WATER DISCHARGE SHALL BE PROVIDED AS SHOWN IN THE HAY-BALE BARRIER DEWATERING DETAIL OR ALTERNATE METHOD PROPOSED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER.
- 9. ALL DISTURBED AREAS SHALL BE RESTORED PER THE SLOPE STABILIZATION AND PERMANENT VEGETATION DETAILS. ALL DISTURBED AREAS THAT ARE SLOPED LESS THAN THREE HORIZONTAL TO ONE VERTICAL (3:1) SLOPE SHALL BE LOAMED, SEEDED, FERTILIZED AND MULCHED PER THE PERMANENT VEGETATIVE COVER SPECIFICATIONS. EROSION CONTROL MATTING SHALL BE PROVIDED ON ALL DISTURBED AREAS THAT ARE SLOPED MORE THAN THREE HORIZONTAL TO ONE VERTICAL (3:1).
- 10. IF FINAL SEEDING OF DISTURBED AREAS IS NOT TO BE COMPLETED BEFORE OCTOBER 15, THE CONTRACTOR SHALL PROVIDE TEMPORARY MULCHING (DORMANT SEEDING MAY BE ATTEMPTED AS WELL) TO PROTECT THE SITE AND DELAY PERMANENT SEEDING.
- 11. WHEN FEASIBLE, TEMPORARY SEEDING OF DISTURBED AREAS THAT HAVE NOT BEEN FINISHED GRADED SHALL BE COMPLETED PRIOR TO OCTOBER 15.
- 12. ANY EROSION WHICH OCCURS WITHIN THE DISTURBED AREAS SHALL BE IMMEDIATELY REPAIRED AND STABILIZED. DURING THE CONSTRUCTION PHASE, INTERCEPTED SEDIMENT SHALL BE RETURNED TO THE SITE. POST SEEDING, INTERCEPTED SEDIMENT, IF ANY, SHALL BE DISPOSED OF IN A MANNER APPROVED BY THE TOWN AND ENGINEER.
- 13. EROSION AND SEDIMENTATION CONTROL MEASURES SHALL REMAIN IN PLACE UNTIL VEGETATION IS RE-ESTABLISHED OR SLOPES ARE STABILIZED AND REMOVAL IS APPROVED BY THE TOWN.
- 14. UNFORESEEN PROBLEMS WHICH ARE ENCOUNTERED IN THE FIELD SHALL BE SOLVED ACCORDING TO THE "2002 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL" BY THE CONNECTICUT COUNCIL ON SOIL AND WATER CONSERVATION IN COOPERATION WITH THE CONNECTICUT
- 15. THE CONTRACTOR SHALL PROVIDE THE NAME AND EMERGENCY CONTACT INFORMATION FOR THE PROJECT PERSONNEL RESPONSIBLE FOR EROSION AND SEDIMENTATION CONTROLS PRIOR TO THE START OF CONSTRUCTION.

NOTE: THE CONTRACTOR SHALL CONTINUALLY STORE THE

FOLLOWING MATERIALS ONSITE DURING CONSTRUCTION

TO MEET UNEXPECTED EROSION NEEDS

* 10 CY OF WOOD CHIPS OR CRUSHED STONE

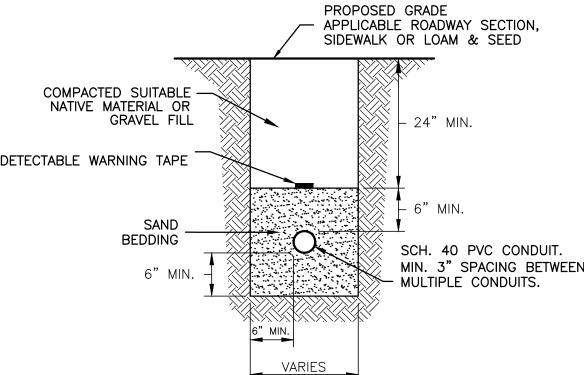
* 100 LF OF SILT FENCE

* 10 HAY BALES

7.5 LBS. PER 1000 S.F. INSPECT SEEDBED BEFORE SEEDING.

ALL DISTURBED AREAS KENTUCKY BLUEGRASS CREEPING RED FESCUE PERENNIAL RYEGRASS

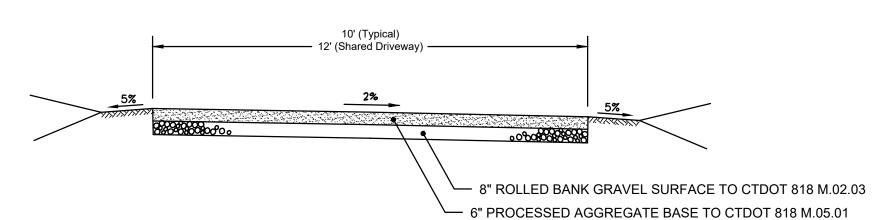
LBS./1000 S.F LBS./ACRE 0.45 0.10 1.00



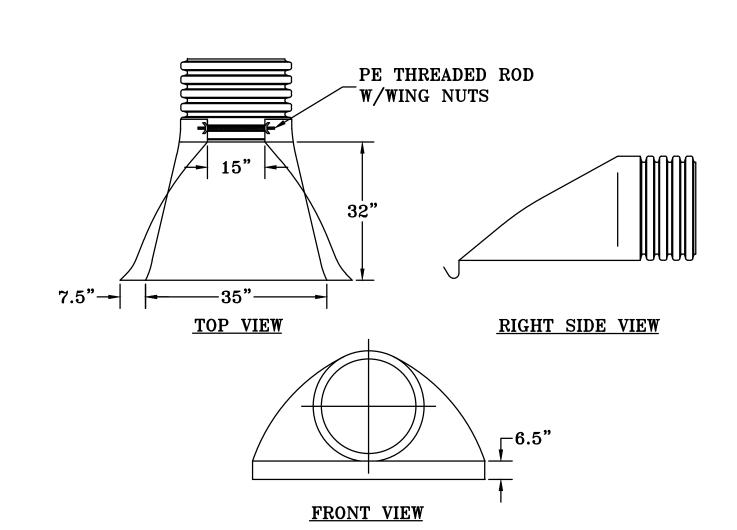
TRENCH DETAIL: ELECTRICAL CONDUIT NOT TO SCALE

<u>PLAN</u>

- DRAINAGE PIPE



TYPICAL DRIVEWAY CROSS SECTION NOT TO SCALE



HDPE FLARED END SECTION

NOT TO SCALE

TEMPORARY VEGETATIVE COVER

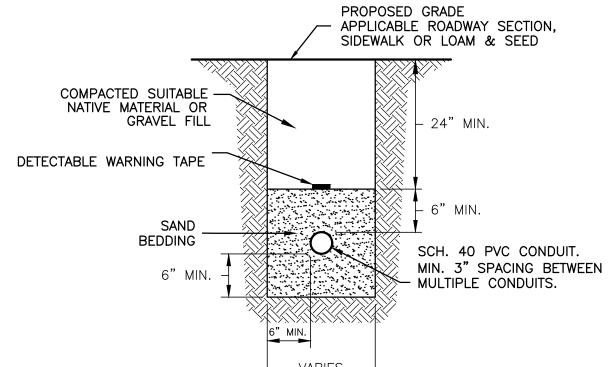
A TEMPORARY SEEDING OF RYE GRASS WILL BE COMPLETED WITHIN 15 DAYS OF THE FORMATION OF STOCKPILES. IF THE SOIL IN THE STOCKPILES HAS BEEN COMPACTED BY CONSTRUCTION OPERATIONS IT SHALL BE LOOSENED TO A DEPTH OF 2 INCHES BEFORE THE FERTILIZER, LIME AND SEED IS APPLIED. 10-10-10 FERTILIZER AT A RATE OF 7.5 POUNDS PER 1000 S.F. LIMESTONE AT A RATE OF 90 LBS. PER 1000 S.F. SHALL BE USED. RYE GRASS APPLIED AT A RATE OF LB. PER 1000 S.F. SHALL PROVIDE THE TEMPORARY VEGETATIVE COVER. STRAW FREE FROM WEEDS AND COARSE MATTER SHALL BE USED AT A RATE OF 70-90 LBS. PER 1000 S.F. AS A TEMPORARY MULCH. APPLY MULCH AND DRIVE TRACKED EQUIPMENT UP AND DOWN SLOPE OVER ENTIRE SURFACE SO CLEAT MARKS ARE PARALLEL TO THE CONTOURS.

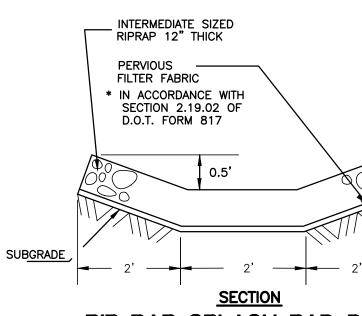
PERMANENT VEGETATIVE COVER

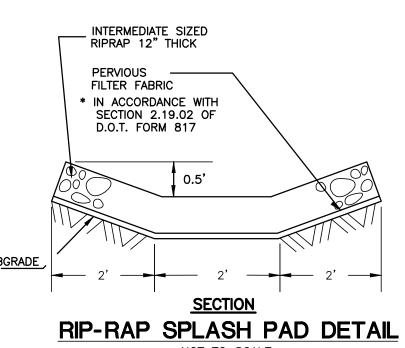
TOPSOIL WILL BE REPLACED ONCE THE EXCAVATIONS HAVE BEEN COMPLETED AND THE SLOPES ARE GRADED AS SHOWN ON THE PLANS. PROVIDE SLOPE PROTECTION AS CALLED FOR ON THE PLANS AND DETAILS. TOPSOIL SHALL BE SPREAD AT A MINIMUM COMPACTED DEPTH OF 4 INCHES. ONCE THE TOPSOIL HAS BEEN SPREAD, ALL STONES TWO INCHES OR LARGER IN ANY DIMENSION WILL BE REMOVED AS WELL AS DEBRIS.

- APPLY AGRICULTURAL GROUND LIMESTONE AT THE RATE OF TWO TONS PER ACRE OR 100 LBS. PER 1000 S.F.
- APPLY 10-10-10 FERTILIZER OR EQUIVALENT AT A RATE OF 300 LBS. PER ACRE OR
- WORK LIMESTONE AND FERTILIZER INTO THE SOIL TO A DEPTH OF 4 INCHES.
- IF TRAFFIC HAS COMPACTED THE SOIL, RETILL COMPACTED AREAS. APPLY THE FOLLOWING GRASS SEED MIX:

TYPICAL SEED MIXTURE







1-3/4낄

6" SOIL COVER -

GRADE

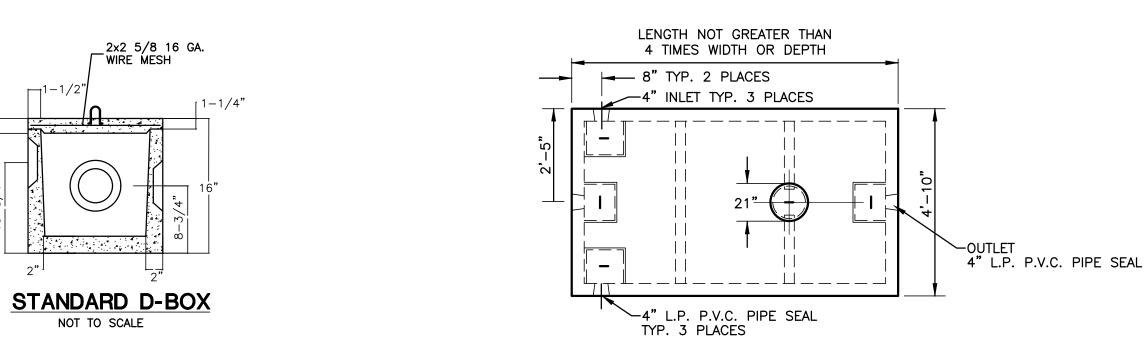
NON-WOVEN FILTER FABRIC

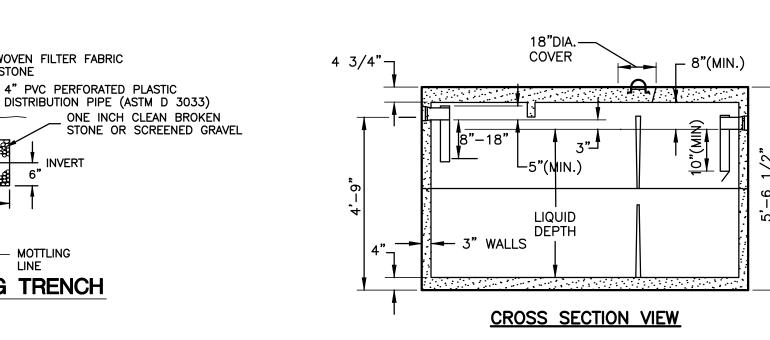
OVER STONE

48" WIDTH

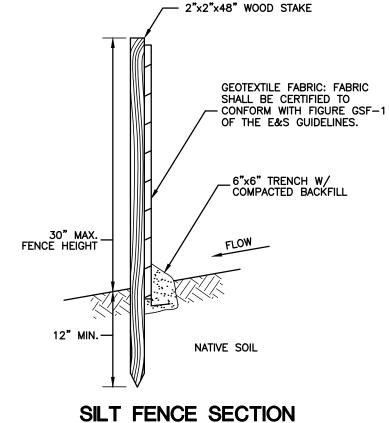
12" x 48" LEACHING TRENCH

NOT TO SCALE

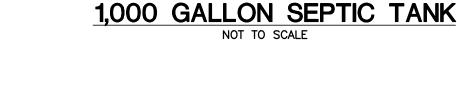


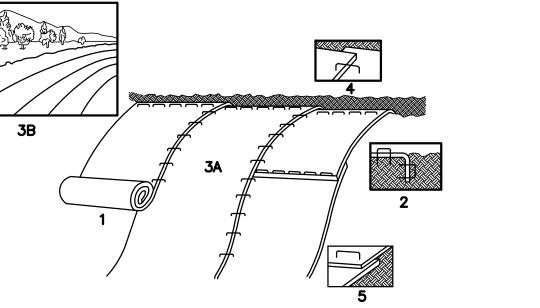


PLAN VIEW 30" MAX. FENCE HEIGHT



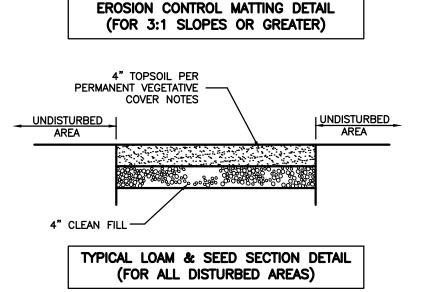
NOT TO SCALE



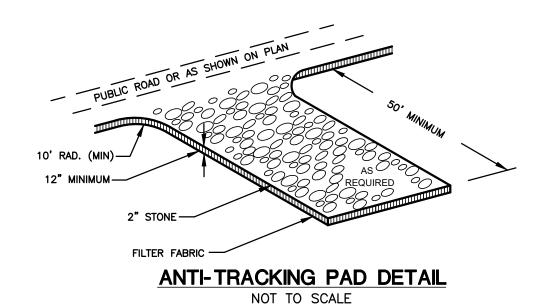


- 1. PROVIDE 4" THICKNESS OF TOPSOIL OVER CLEAN FILL. PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING APPLICATION OF LIME, FERTILIZER, AND SEED MIX PER PERMANENT VEGETATIVE COVER NOTES. (SHALL BE PAID FOR AT THE UNIT PRICE FOR LOAM, SEED, FERTILIZE & MULCH) BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN 6" DEEP x 6" WIDE TRENCH, BACKFILL AND COMPACT THE TRENCH AFTER STAPLING.
- ROLL THE BLANKET (A) DOWN OR (B) HORIZONTALLY ACROSS THE SLOPE. 5. WHEN BLANKETS MUST BE SPLICED DOWN THE SLOPE, PLACE BLANKETS END OVER END (SHINGLE STYLE) WITH APPROXIMATELY 4" OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELÝ 12" APART.

NOTE: ALL PERMANENT EROSION CONTROL BLANKETS ARE TO BE NORTH AMERICAN GREEN BIONET C125BN OR APPROVED EQUAL.



SLOPE STABILIZATION DETAILS NOT TO SCALE



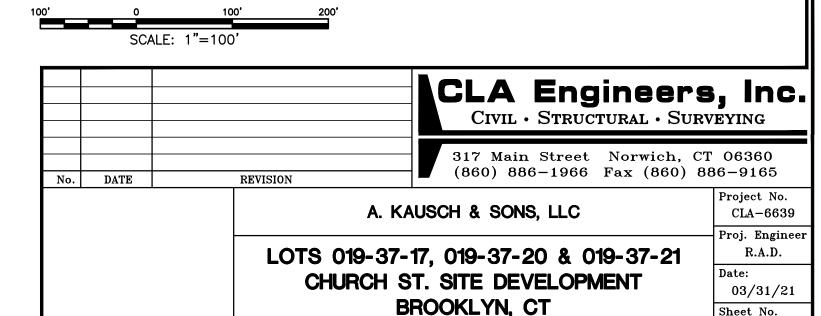
-DRIVEWAY SECTION 6" MIN. ~ PRESHAPE BEDDING DRAINAGE PIPE TO FIT PIPE CONTOUR 6" MINIMUM 12" MINIMUM OVER EARTH OVER ROCK GRANULAR FILL CTDOT 818 M.02.01

> DRAINAGE PIPE BEDDING DETAIL NOT TO SCALE

NOTES & CONSTRUCTION DETAILS

Sheet No.

3.5' TRENCH WIDTH



DRAINAGE NARRATI'VE

3-Lot Subdivision Church Street, Brooklyn, CT Prepared for Kausch & Sons, LLC

The existing parcels consist of a total of approximately 27 acres of undeveloped woodlands located to the west of Church Street in Brooklyn Connecticut. There are inland wetlands located in the north and southern parts of the site.

The proposed development consists of 2 residential building lots served by approximately 950 L.F. of new shared driveway access from Church Street. Presently, storm water in the proposed development area drains north to south, exiting the site via the wetlands and eventually discharging to the Quinebaug River to the east.

The shared driveway for the building lots is required to cross existing wetlands in three locations. The crossing locations have been determined to minimize impact to the wetland. The crossing lengths are approximately 50, 75 and 73 feet respectively.

The following determines the size of the drainage culverts required to pass the 25-year storm event with inlet control.

Methodology:

In accordance with the Town of Brooklyn's Public Improvement Specifications, the site's watershed was analyzed using the Rational method for the 25-year storm. The Rational method predicts the peak runoff according to the formula: Q=CiA, where C is a runoff coefficient, i is the rainfall intensity, and A is the sub-catchment area.

Rainfall intensities used in the calculations were taken from the Brooklyn (06-0918) weather station readings accessed via the NOAA Atlas 14 Point Precipitation Frequency website.

DEEP watershed basin boundaries and Connecticut Elevation (Lidar) Data (See SK-1) was used to determine the approximate watershed area contributing to each driveway crossing.

The site consists primarily undeveloped woodlands. A run-off coefficient (C) of 0.2 (Unimproved Surface) was utilized. The Time of Concentration for each catchment was determined using the TR-55 method.

The peak discharge (Q) for the 25-year storm event was calculated as follows:

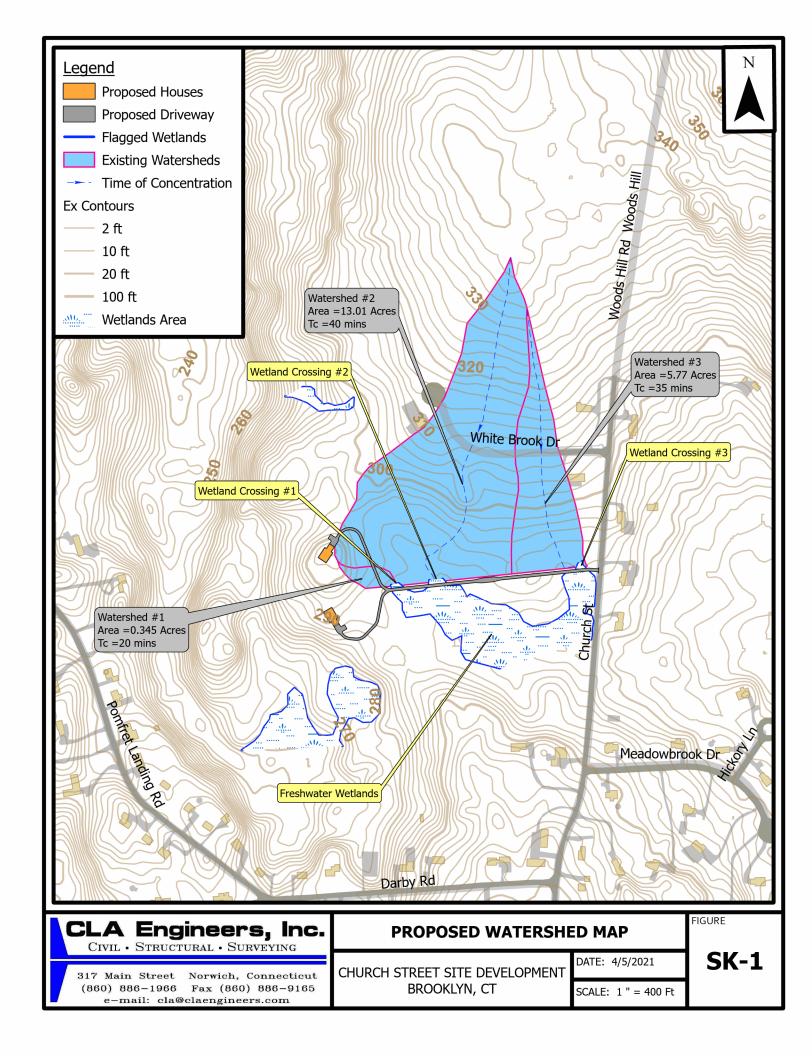
Peak Volume (Q) = $CiA = 0.2 \times 6.11 \text{ in/hr } \times Area (acres)$

Analysis of each culvert crossing was performed using Hydraflow Express culvert modeler (used in HDS-5 Hydraulic Design of Highway Culverts).

The resultant analysis determined the size and number of culverts required to be installed at a grade consistent with the existing wetland (See Appendix 2).

The following table presents the results for each crossing:

Location	Watershed Area	Tc (Mins)	Peak Volume (cfs	Pipe Required
Crossing 1	0.34	20	0.37	1 x 15"
Crossing 2	13.01	40	9.7	3 x 15"
Crossing 3	5.77	35	4.6	1 x 15"



Hydrograph Report

APPPENDIX

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Apr 5, 2021

Hyd. No. 1

Wetland Crossing 1

Hydrograph type = Rational Storm frequency = 25 yrs Time interval = 1 min Drainage area = 0.345 ac

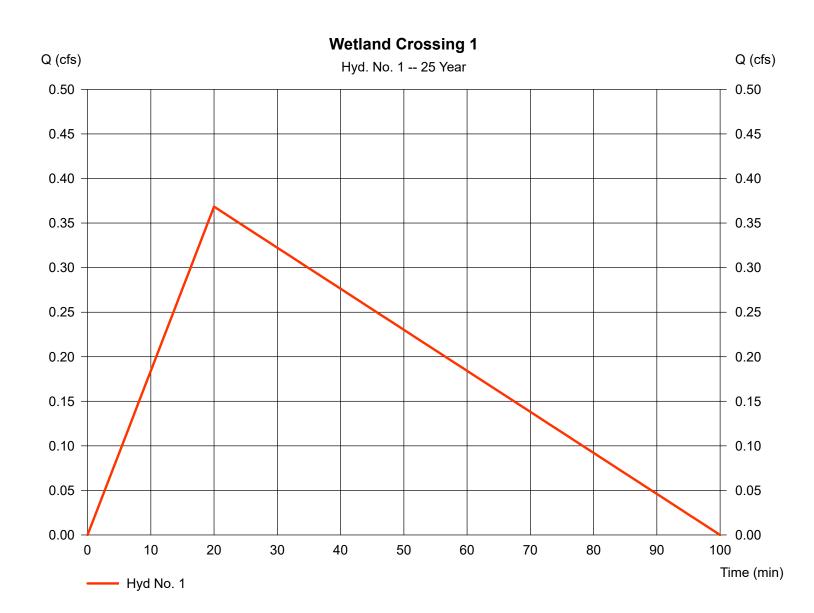
Intensity = 5.339 in/hr

= 6639 Church_St.IDF **IDF** Curve

Peak discharge = 0.368 cfsTime to peak = 20 min Hyd. volume = 1,105 cuftRunoff coeff. = 0.2

Tc by TR55 = 20.00 min

Asc/Rec limb fact = 1/4



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Apr 5, 2021

Hyd. No. 2

Wetland Crossing 2

Hydrograph type = Rational
Storm frequency = 25 yrs
Time interval = 1 min
Drainage area = 13.010 ac
Intensity = 3.728 in/hr

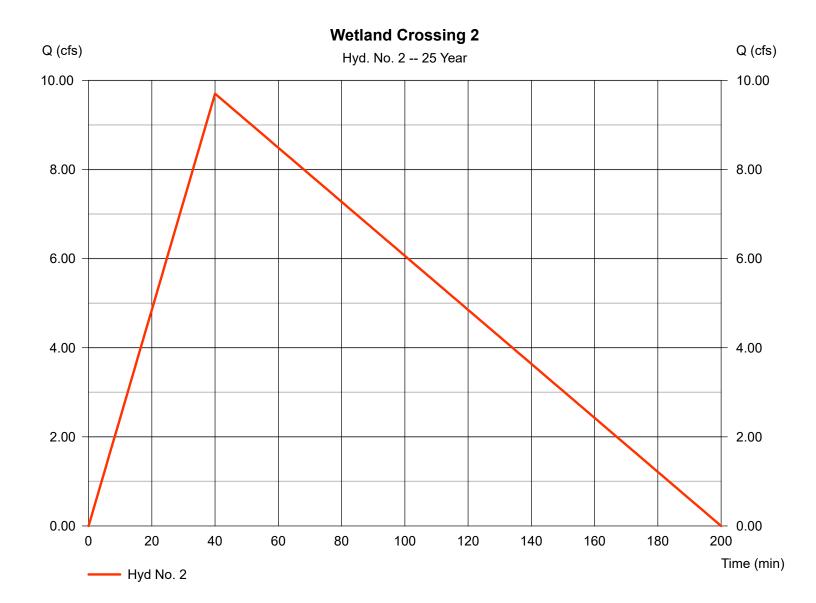
IDF Curve = 6639 Church_St.IDF

Peak discharge = 9.701 cfs
Time to peak = 40 min
Hyd. volume = 58,207 cuft

Runoff coeff. = 0.2

Tc by TR55 = 40.00 min

Asc/Rec limb fact = 1/4



Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc. v6.066

Monday, Apr 5, 2021

Hyd. No. 3

Wetland Crossing 3

Hydrograph type = Rational
Storm frequency = 25 yrs
Time interval = 1 min
Drainage area = 5.770 ac
Intensity = 4.023 in/hr

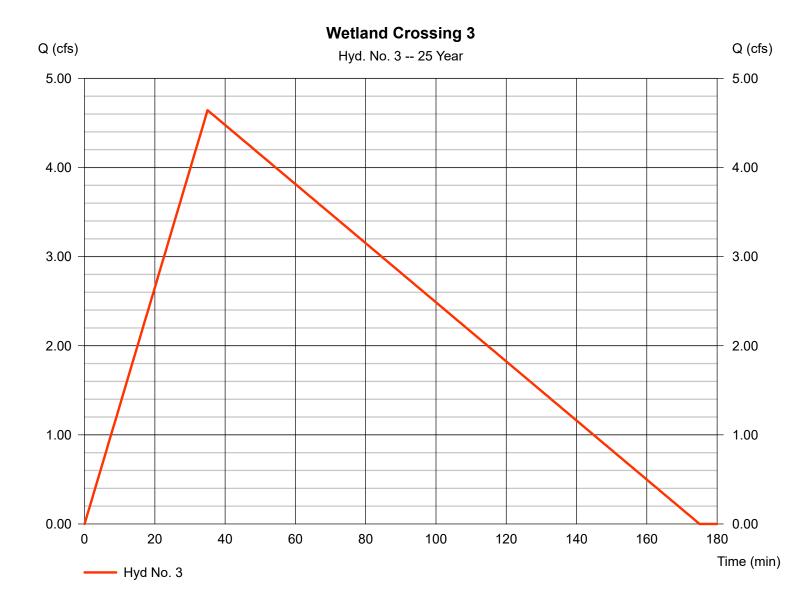
IDF Curve = 6639 Church_St.IDF

Peak discharge = 4.643 cfs
Time to peak = 35 min
Hyd. volume = 24,375 cuft

Runoff coeff. = 0.2

Tc by TR55 = 35.00 min

Asc/Rec limb fact = 1/4



Culvert Report

Hydraflow Express Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc.

Tuesday, Apr 6 2021

Wetland Crossing 1

Invert Elev Dn (ft) = 287.80Pipe Length (ft) = 24.00Slope (%) = 0.21Invert Elev Up (ft) = 287.85Rise (in) = 15.0Shape = Cir = 15.0Span (in) No. Barrels = 1 n-Value = 0.012Inlet Edge = Projecting Coeff. K,M,c,Y,k = 0.0045, 2, 0.0317, 0.69, 0.5

Embankment

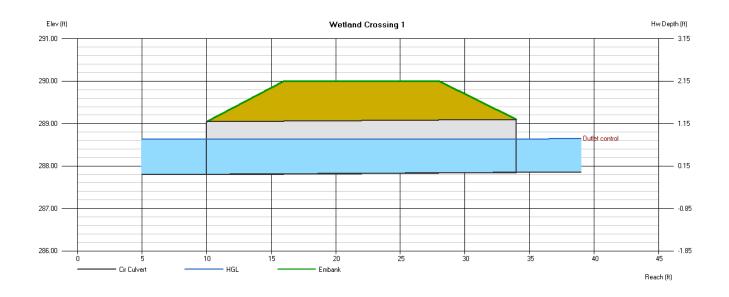
Top Elevation (ft) = 290.00 Top Width (ft) = 12.00 Crest Width (ft) = 50.00 **Calculations**

Qmin (cfs) = 1.00 Qmax (cfs) = 1.00 Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 1.00 Qpipe (cfs) = 1.00 Qovertop (cfs) = 0.00Veloc Dn (ft/s) = 1.17Veloc Up (ft/s) = 1.24HGL Dn (ft) = 288.62HGL Up (ft) = 288.63Hw Elev (ft) = 288.64 Hw/D (ft) = 0.63

Flow Regime = Outlet Control



Hydraflow Express Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc.

Wetland Crossing 2

Invert Elev Dn (ft) = 285.90Pipe Length (ft) = 24.00Slope (%) = 0.83Invert Elev Up (ft) = 286.10Rise (in) = 15.0Shape = Cir = 15.0Span (in) = 2 No. Barrels = 0.012n-Value Inlet Edge = Projecting Coeff. K,M,c,Y,k = 0.0045, 2, 0.0317, 0.69, 0.5

Embankment

Top Elevation (ft) = 288.40 Top Width (ft) = 12.00 Crest Width (ft) = 50.00

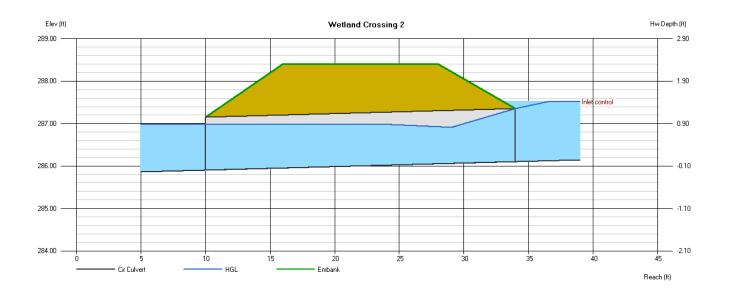
Calculations

Qmin (cfs) = 5.00Qmax (cfs) = 15.00Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 10.00 Qpipe (cfs) 10.00 Qovertop (cfs) = 0.00Veloc Dn (ft/s) = 4.44Veloc Up (ft/s) = 5.21HGL Dn (ft) = 286.98HGL Up (ft) = 287.01Hw Elev (ft) = 287.51Hw/D (ft) = 1.13

Flow Regime = Inlet Control



Hydraflow Express Extension for AutoCAD® Civil 3D® 2009 by Autodesk, Inc.

Wetland Crossing 3

Invert Elev Dn (ft) = 287.80Pipe Length (ft) = 24.00Slope (%) = 6.25Invert Elev Up (ft) = 289.30Rise (in) = 15.0Shape = Cir = 15.0Span (in) No. Barrels = 1 n-Value = 0.012Inlet Edge = Projecting Coeff. K,M,c,Y,k = 0.0045, 2, 0.0317, 0.69, 0.5

Embankment

Top Elevation (ft) = 291.00 Top Width (ft) = 12.00 Crest Width (ft) = 50.00

Calculations

Qmin (cfs) = 1.00 Qmax (cfs) = 10.00 Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 5.00 Qpipe (cfs) = 5.00 Qovertop (cfs) = 0.00Veloc Dn (ft/s) = 4.44Veloc Up (ft/s) = 5.22HGL Dn (ft) = 288.88HGL Up (ft) = 290.21Hw Elev (ft) = 290.71Hw/D (ft) = 1.13

Flow Regime = Inlet Control

